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GOLF COURSE SUBDIVISION UNIT NO. 2
PRELIMINARY SOIL REPORT

MAUNALUA, OAHU, HAWAII
TAX MAP KEY: 3-9-11

To:
KAISER-AETNA

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

SEPTEMBER 19, 1970

MUNICIPAL REFERENCE & RECORDS CENTER
City & County of Honolulu

City Hall Annex - 155 S. King Street
Honolulu, Hawaii 96813

WITHDRAWN

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

WALTER LUM
EDWARD WATANABE
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3030 WAIALAE AVE., HONOLULU, HAWAII 96816 • TEL. 737-7931

September 19, 1970

KAISER-AETNA
P. O. Box 2997
Honolulu, Hawaii 96802

Gentlemen:

Subject: Golf Course Subdivision Unit No. 2
Preliminary Soil Report
(for site grading design purposes)
Tax Map Key: 3-9-11
Chapter 23, Revised Ordinances of
Honolulu, 1961 As Amended

The Resort Division area consists of resort, apartment and residential subdivisions.

In accordance with your request, preliminary soil explorations were made to cover the general area. This report concerns only the preliminary soil explorations at the site for the proposed Golf Course Subdivision Unit No. 2, Maunalua, Oahu, Hawaii.

The borings generally indicated tan and brown sandy and clayey silts to about 10 to 20 ft, the depths drilled. Black sand was encountered in Boring Nos. 25 and 26 below 15 ft. Boulders or rock was encountered in Boring Nos. 29 and 34 below 15 ft.

A low ridge extending from Koko Crater cuts across the west side of the site.

Some grading and filling of the site are contemplated. The earthwork should be done in accordance with the requirements of Chapter 23, Revised Ordinances of Honolulu, 1961 As Amended and the recommendations contained herein.

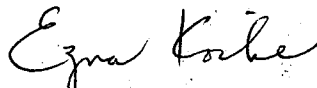
Light apartment structures may be constructed with ordinary footings or foundations.

High-rise buildings in the area will probably require relatively simple foundations. However, additional explorations should be made for the design of a specific structure and location.

The report includes a Boring Location Plan, boring logs, laboratory test results, recommendations and limitations.

Respectfully submitted,

WALTER LUM ASSOCIATES, INC.



Ezra Koike
Professional Engineer
Hawaii No. 1450

EK:rmf

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GOLF COURSE SUBDIVISION UNIT NO. 2
PRELIMINARY SOIL REPORT

MAUNALUA, OAHU, HAWAII
TAX MAP KEY: 3-9-11

SCOPE OF EXPLORATION

The Resort Division area consists of resort, apartment and residential subdivisions. This report concerns only the preliminary soil explorations at the site for the proposed Golf Course Subdivision Unit No. 2 at Maunaloa, Oahu, Hawaii. The limits of this area are shown on Figure 1. The purpose of this exploration was to determine general soil conditions for site grading.

This report includes field exploration, laboratory tests and general recommendations for site grading and light building foundation design.

FIELD EXPLORATION

Twelve borings were made at the site. The locations of these borings are shown on Figure 1, Boring Location Plan.

The borings were made with 3 and 4-in. diameter augers using tungsten carbide bits. Soil samples were recovered with a standard split spoon driven with a 140-lb hammer falling 30 inches.

Soil samples were visually observed and subjected to appropriate tests in the laboratory. Based on visual observations and laboratory tests, the soil descriptions in the boring logs are generally made in accordance with the "Unified Soil Classification System."

LABORATORY TESTS

Laboratory tests for on-site soils included: natural water contents, Atterberg limits, specific gravity, sieve analysis, AASHTO T-180-57 density, expansion and CBR.

A list of the standard field and laboratory test methods used for this project is given in the Appendix.

A summary of the laboratory test results is given in Table IA.

GENERAL SITE CONDITIONS

The proposed subdivision site is about 1,500 ft north of Kalaniana'ole Highway and the center of the site is generally west of Ehukai Street.

The site generally slopes downward in a southeasterly direction at about 2 to 5% gradient.

A low ridge extending from the rim of Koko Crater cuts across the west side of the site.

Houses, sheds and pig pens were located along both sides of Ehukai Street. Pig farms were previously located in this area and a nursery was located on the west side of Ehukai Street.

At the time of the field exploration, ground cover consisted mostly of kiawe and koa with some areas of tall grass. Subsequent fires have burned off most of the growth and many of the existing structures.

INTERPRETATION OF SOIL CONDITIONS

From the field exploration, the soils at the site may be described as follows:

Tan and brown sandy and clayey silts with mudrock to about 10 to 20 ft, the depths drilled. Black sand was encountered in Boring Nos. 25 and 26 at about 15-ft depth. Boulders or rock was encountered in Boring Nos. 29 and 34 at about 15-ft depth.

Water was not noted in the borings during the field explorations.

For more detailed descriptions of soils encountered in the drill holes, refer to the boring logs.

DISCUSSION AND RECOMMENDATIONS

The proposed plan is to grade the site for apartment development with cuts and fills generally less than about 10 to 15 ft in height.

Site Grading

All surface vegetation and miscellaneous debris should be cleared and removed prior to site filling. Localized soft pockets encountered during site preparations should be

excavated and backfilled with compacted select material. Provisions to drain the site should be included during and after the completion of filling operations.

Grading work should be done in general conformance with the requirements of Chapter 23, Revised Ordinances of Honolulu, 1961 As Amended.

For the construction of fills, the following is recommended:

1. Rubble, loose boulders and unsuitable materials should be removed.
2. Old cesspools should be accurately located on the grading plan and backfilled before any grading work is started.
3. Hard surfaces along existing access roads should be scarified down to stiff soils and recompactd to match the density of the surrounding soils.
4. Loose surface soils along the sides and bottom of natural drainageways should be removed where fills are contemplated.

Subdrains should be placed in a herringbone pattern along the bottom of natural drainageways or dips before the placement of fills.

5. Fill material may be approved on-site or borrow soils. If practicable, fill material imported to the site should be select soils with the plasticity index generally less than 20.
6. Fills should be constructed in approximately level layers starting at the lower end and working upward. Where fills are made on sloping areas steeper than about 5 horizontal to 1 vertical, the ground at the toe of the fill should be benched to a generally level condition. As the fill is brought up, it should be continually keyed into the stiff natural ground by cutting steps into the slopes and compacting the fill into these steps.
7. Fills should be laid in 6-in. compacted layers with a relative density of at least 90% of AASHO T-180-57 density.
8. If boulders are proposed to be used in the construction of fills, they should generally be placed along the toe sections of fill slopes and outside of probable building sites.

Before placing any boulders, the subgrade should be stripped to stiff natural ground and shaped to drain. A layer of granular filter material should be placed on the subgrade and the boulders placed on the filter layer. The void spaces between boulders should be filled with granular material. A blanket of filter material should be placed against the boulders before any earth fills are placed against the boulders. See attached sketch, Figure 2.

Slopes

In general, cut and fill slopes of 2 horizontal to 1 vertical or flatter should be used.

If slope heights (top to toe) of greater than 15 ft are considered, 8-ft wide benches should be placed at height intervals of about 15 ft in both cuts and fills.

For protection against erosion, the runoff from rainstorms should be diverted by berms or ditches away from slopes whenever practicable.

The surface of fill slopes should be compacted by cat-tracking or with a sheepsfoot roller.

In general, slope planting is recommended on cut and fill slopes to minimize erosion.

Foundations

Light, short-span apartment structures may be constructed at the site with ordinary footings or foundations.

High-rise buildings in the area will probably require relatively simple foundations. However, additional explorations should be made for the design of a specific structure and location.

For heavy or long-span or multiple-story structures, foundation explorations should be made at each building site to evaluate the ground conditions before foundations are designed.

The following may be used as a guide for foundation design for light, 2 to 3-story light structures:

1. Bearing values for a given soil vary with the size and depth of footings. For light, 2 to 3-story, short-span structures, bearing values of about 2000 p.s.f. may be used.
2. Any portion of a building that is over an old cesspool should be designed to span the cesspool.
3. If soft spots or pockets of loose material are encountered in footing excavations or below a

building area, they should be excavated and replaced with compacted select on-site or borrow soils.

4. Concrete slab on ground should be placed over a base course of 4 in. of well-graded gravel less than 3/4 in. and greater than 1/4 in. in size. The subgrade should be compacted and shaped to a level surface or to drain, if practicable, and generally should be kept slightly higher than the finish grade outside of the building.
5. In general, buildings and structures should be placed about 15 ft from the tops of slopes.
6. Construction of retaining walls on slopes should generally be avoided.
7. Good surface drainage away from the foundation of structures should be maintained and the site should be graded at all times to prevent ponding of water.

Roadway

In general, a rough estimate of the roadway pavement thickness for the light residential traffic anticipated is as follows:

1. Wearing course - 2-in. asphaltic concrete.
2. Base course - 6-in. base course over a prepared subgrade.

Provisions should be made in the contract documents to allow for local adjustments regarding subbase requirements in the field as ground conditions are exposed at subgrade levels. The subbase thickness will depend upon the type of material within the top 2 ft of subgrade.

The subgrade should be compacted and shaped to drain. To avoid the ponding of water and softening of the subgrade at low points, weep holes should be placed at subgrade levels through the walls of catch basins which are placed in these low areas.

Utilities

Although the probability of differential settlements in localized areas is slight in this area, utilities should be placed after the fills are constructed. Utility lines should be designed with flexible joints, particularly where lines are connected to structures. Gravity flow lines should be made as steep as practicable.

Unforeseen or undetected conditions may occur in localized areas and will have to be adjusted and corrected in the field as they are detected.

PROPOSED SPECIFICATION FOR EARTHWORK

GOLF COURSE SUBDIVISION UNIT NO. 2

General Description

This item shall consist of clearing and grubbing, removing of existing structures, preparing of land to be filled, excavating and filling of the land, spreading, compacting and testing of the fill, and subsidiary work necessary to complete the grading.

Clearing, Grubbing and Preparing Areas to be Filled

Vegetation, concrete slabs and rubbish shall be removed and disposed of, leaving the disturbed area with a neat, debris-free appearance.

Vegetable matter shall be removed from the surface upon which fill is to be placed. Topsoil and stockpiled soils shall be (1) stripped to stiff natural ground or (2) scarified and recompact before the placement of fills. Topsoil encountered at finish grade shall be scarified and recompact.

Hard surfaces along the existing access roads shall be scarified down to stiff soils and recompact to match the density of the surrounding soil before the placement of fills.

Cesspools shall be flagged in the field and accurately located on the plans and backfilled before any grading work is started. The procedure for backfilling of cesspools follows in the section "Backfilling of Old Cesspools".

Where fills are proposed in sidehill areas and gullies, loose material along the bottom and the sides shall be stripped down to stiff natural ground before the placement of fills. New fills shall be keyed into the stiff natural ground.

Subdrains shall be placed along the bottom and sides of the natural drainageways before the construction of fills. The locations of subdrains should be determined in the field after clearing and grubbing.

Where fills are made on sloping areas steeper than 5 horizontal to 1 vertical, the ground at the toe of the slope shall be benched to a generally level condition. As the fill is brought up, it shall be continually keyed into the stiff natural ground by the cutting of steps into the hillside and compacting the fill into these steps. Ground slopes which are flatter than 5 horizontal to 1 vertical shall be benched when considered necessary by the Soil Engineer.

Materials

Fill materials shall consist of approved on-site or borrow soils. The soils shall contain no more than a trace of organic matter. Fill material imported to the site shall be select soils with a plasticity index less than 20.

Placing, Spreading and Compacting Fill Material

The selected fill material shall be placed in level layers which, when compacted, shall not exceed 6 inches. Each layer shall be spread evenly and thoroughly blade-mixed during the spreading to insure uniformity of material and water content within each layer.

No rocks or cobbles shall be allowed to nest and voids between rocks must be carefully filled and compacted with small stones or earth.

When the water content of the fill material is well below the optimum for compacting purposes, water shall be added until the water content assures a thorough bonding during the compacting process.

When the water content of the material is well above the optimum for compacting purposes, the fill material shall be aerated by blading or by other satisfactory methods until the water content is near the optimum.

After each layer has been placed, mixed and spread evenly, it shall be thoroughly compacted to no less than 90% of maximum density in accordance with AASHTO Test No. T-180-57 or other comparable density tests. Compaction shall be with sheepfoot rollers, multiple-wheel pneumatic-tired rollers or other acceptable rollers which shall be able to compact the fill to the specified density. Rolling shall be accomplished while the fill material is near the optimum water content. The rolling of each layer shall be continuous over its entire area and the roller shall make sufficient passes to insure the obtainment of the desired density.

Field density tests shall be made to get an indication of the compaction of the fill. Where sheepfoot rollers are used, the soil may be disturbed to a depth of several inches. Density readings shall be taken as often as necessary in the compacted material below the disturbed surface. When these readings indicate that the density of any layer of fill or portion thereof is below the required 90% density, that layer or portion shall be reworked until the required density has been obtained.

The fill operation shall be continued in 6-in. compacted layers as specified above, until the fill has been brought to the finished slopes and grades as shown on the accepted plans.

Backfilling of Old Cesspools

The following procedures shall be followed for backfilling:

(1) Sludge Removal

Remove the sludge from the bottom of the old cesspool by (a) pumping or (b) by clamshell or any other suitable way. The material shall be disposed of away from the site. The completeness of removal shall be verified by probing and shall be less than 12 in. at the bottom.

(2) Granular Fill (below 3 ft from finish grade)

Use granular material, graded from 6 to 0 inches. The fines passing the No. 200 sieve shall be less than 10%. The materials shall be placed in thin layers (12 in. maximum) and compacted with vibratory equipment to 90% of AASHTO T-180-57 density. Ramming each layer into place with a clamshell bucket will be allowed. The granular fill shall be wetted before placement into the cesspools. Sufficient compaction tests shall be conducted to verify that 90% compaction is obtained by the construction method selected.

(3) Top 3 Ft of Fill

Linings encountered in the cesspools within the top 3 ft from finish grade shall be removed. The fill within the top 3 ft from finish grade shall be constructed from on-site soil in thin layers (6-in. compacted thickness) to 90% of AASHTO T-180-57 density. The material at finish grade shall blend with the surrounding soil.

Excavation

Suitable material from excavation shall be used in the fill and unsuitable material from excavation shall be disposed of.

Boulder Fills

If boulders are proposed to be used in the construction of fills, they shall be placed along the toe section of slopes and at locations indicated on the plan. The subgrade shall be stripped to stiff natural ground and shaped to drain. A layer of granular filter material shall be placed on it. All voids between boulders shall be filled with smaller granular soils. A blanket of filter material shall be placed against the boulder fill before construction of earth fills behind or above the boulders.

Unforeseen Conditions

If unforeseen or undetected critical soil conditions such as soft spots are encountered during the field operation, corrective measures shall be made in the field as they are detected.

Rainy Weather

No fill material shall be placed, spread or rolled during unfavorable weather conditions. When the work is interrupted by heavy rain, fill operations shall not be resumed until field tests indicate that the water content and density are as previously specified.

BORING LOGS

Symbols

Symbols used generally are in accordance with the Unified Soil Classification System.

Where a parenthesis "(MH)" is used, the soil sample was classified by visual observation of the sample recovered.

Where no parenthesis "MH" is used, the soil sample was classified from either the Atterberg limits or sieve analysis test results.

WALTER LUM ASSOCIATES

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii
Tax Map Key: 3-9-11

HAMMER:

Weight 140 #

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 22 Sheet No. _____ of _____

Driller WALTER LUM ASSOC. Date JULY 13, 1970

Field Party HASHIDA, MAESHIRO

Type of Boring AUGER (B-30-L) Diam. 4"

Elev. 142' ± * Datum —

Drill Bit T.C. DRAG

Water Level NOT NOTED

Time —

Date 7-13-70

PENETRATION DATA

Standard
Penetration Test

N (Blows per foot)

0 10 20 30 40

Unified
Soil
Classification

DESCRIPTION

ELEV. = 142' ± *

Depth (ft.)

Sampler

Sample No.

Wet Dens.
P.C.F.

Water Cont.
%

Dry Dens.
P.C.F.

Unconf. Comp.
P.S.F.

Vane Shear
P.S.F.

SM

MEDIUM DENSITY, BROWN,
SILTY SAND
W/MUDROCK FRAGMENTS

0



22-A

15

(SM)

MEDIUM DENSITY, BROWN
SILTY SAND
W/MUDROCK FRAGMENTS

5



22-B

16

(SM-GM)

MEDIUM DENSITY, TAN
SILTY SAND
W/GRAVEL & COBBLES
(MUDROCK)

10



22-C

18

(ML)

STIFF, TAN
SANDY SILT
W/MUDROCK FRAGMENTS

15



22-D

28

(ML)

STIFF, BROWN
FINE SANDY SILT
W/MUDROCK FRAGMENTS

20



22-E

30

END OF BORING @ 21.0'

*
ELEVATION ESTIMATED
FROM CONTOUR PLAN

30/5'

20/5'

WALTER LUM ASSOCIATES

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Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaulua, Oahu, Hawaii

Tax Map Key: 3-9-11

HAMMER:

Weight 140*

Drop 30"

SAMPLER:

2" STANDARD SPLIT SPOON

BORING NO. 25 Sheet No. _____ of _____

Driller WALTER LUM ASSOC. Date JULY 14, 1970

Field Party HASHIDA, MAESHIRO

Type of Boring AUGER (B-30-L) Diam. 4"

Elev. 85' ± * Datum _____

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time _____

Date 7-14-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)

0 10 20 30 40

Unified Soil Classification	DESCRIPTION	Depth (Ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	Standard Penetration Test
	ELEV. = 85' ± *	0								
(ML)	STIFF, TAN, CLAYEY SILT			25-A	-	14	-	-	-	
(ML)	STIFF, BROWN, CLAYEY SILT & DECOMPOSED MUDROCK	5		25-B	-	24	-	-	-	35/5'
(ML)	STIFF, TAN, SANDY SILT (DECOMPOSED MUDROCK)	10		25-C	-	22	-	-	-	25/5'
(SP-SM)	DENSE, BLACK, SAND & TRACE OF CORAL	15		25-D	-	8	-	-	-	
	END OF BORING @ 21.5'	20		25-E	-	7	-	-	-	

*

ELEVATION ESTIMATED FROM CONTOUR PLAN

WALTER LUM ASSOCIATES

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Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii

Tax Map Key: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 26 Sheet No. _____ of _____

Driller WALTER LUM ASSOC. Date JULY 14, 1970

Field Party PANG, MAKAULA

Type of Boring AUGER (ACKER ACE) Diam. 4"

Elev. 83' ± * Datum _____

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time _____

Date 7-14-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)

0 10 20 30 40

Unified Soil Classification

DESCRIPTION

ELEV. = 83' ± *

Depth (ft.)

Sampler

Sample No.

Wet Dens. P.C.F.

Water Cont. %

Dry Dens. P.C.F.

Unconf. Comp. P.S.F.

Vane Shear P.S.F.

Unified Soil Classification	DESCRIPTION	Depth (ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	Standard Penetration Test N (Blows per foot)
(ML)	STIFF, TAN CLAYEY SILT w/ SAND & GRAVEL (DECOMPOSED MUDROCK)	0 - 5		26-A	-	22	-	-	-	5
(ML)	STIFF, BROWN, CLAYEY SILT w/ DECOMPOSED MUDROCK & GRAVEL	5 - 10		26-B	-	27	-	-	-	55
(SP-SM)	DENSE, BLACK SILTY SAND	10 - 15		26-C	-	39	-	-	-	35/5
	ROCK	15 - 20		26-D	ROCK FRAGMENTS					56/3
	END OF BORING @ 21.5'	20 - 21.5		26-E	-	7	-	-	-	

* ELEVATION ESTIMATED FROM CONTOUR PLAN

WALTER LUM ASSOCIATES

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Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii
Tax Map Key: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 27 Sheet No. _____ of _____

Driller WALTER LUM ASSOC. Date JUNE 13, 1970

Field Party ASATO, MAESHIRO

Type of Boring AUGER (MOBILE MINUTEMAN) Diam. 3"

Elev. 96' ± * Datum _____

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time _____

Date 6-13-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)
0 10 20 30 40

Unified Soil Classification

DESCRIPTION

ELEV. = 96' ± *

Depth (Ft.)

Sampler

Sample No.

Wet Dens. P.C.F.

Water Cont. %

Dry Dens. P.C.F.

Unconf. Comp. P.S.F.

Vane Shear P.S.F.

Unified Soil Classification	DESCRIPTION	Depth (Ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	N (Blows per foot)
ML	MEDIUM, TAN BROWN SANDY SILT w/MUDROCK	0		27-A	-	15	-	-	-	
		5		27-B	-	21	-	-	-	
(ML)	STIFF, TAN, CLAYEY SILT w/MUDROCK	10		27-C	-	28	-	-	-	32/5
	END OF BORING @ 11'									

* ELEVATION ESTIMATED FROM CONTOUR PLAN

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

Boring Log

GOLF COURSE
PROJECT SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii
Tax Map Key: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 28 Sheet No. _____ of _____

Driller WALTER LUM A440C. Date JUNE 12, 1970

Field Party LUNING, MAESHIRO

Type of Boring AUGER (MOBILE MINUTEMAN) Diam. 3"

Elev. 61' ± * Datum

Drill Bit T.C. DRAG

Water Level NOT NOTICED			
-------------------------	--	--	--

Time	—				
------	---	--	--	--	--

Date: 6-12-70				
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Unified Soil Classification	DESCRIPTION	Depth (Ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	PENETRATION DATA				
										Standard Penetration Test				
										N (Blows per foot)				
										0	10	20	30	40
(ML)	ELEV. = 61' ± 2*	0												
	MEDIUM TO STIFF TAN, SANDY SILT	5		28-A	-	18	-	-	-					
				28-B	-	16	-	-	-				261.5'	
	END OF BORING @ 11.0'	10		28-C	-	30	-	-	-				291.5'	
* ELEVATION ESTIMATED FROM CONTOUR PLAN														

WALTER LUM ASSOCIATES

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Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii

Tax Map Key: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER:

2" STANDARD SPLIT SPOON

BORING NO. 20 Sheet No. _____ of _____

Driller WALTER LUM ASSOC. Date JUNE 8, 1970

Field Party ASATO, MAKAULA

Type of Boring AUGER (CONCORE) Diam. 4"

Elev. 54' ± * Datum _____

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time _____

Date 6-8-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)
0 10 20 30 40

Unified Soil Classification

DESCRIPTION

ELEV. = 54' ± *

Depth (ft.)

Sampler

Sample No.

Wet Dens. P.C.F.

Wet Cont. %

Dry Dens. P.C.F.

Unconf. Comp. P.S.F.

Vane Shear P.S.F.

MH

MEDIUM DENSITY, BROWN
CLAYEY SILT w/ SAND
{ GRAVEL
(DECOMPOSED MUDROCK)

5

20-A

22

24/5'

(ML)

DENSE, TAN,
SANDY SILT w/
DECOMPOSED MUDROCK

10

20-B

17

44

15

20-C

24

36/5'

ROCK

20-D

30

HAMMER
BOUNCES

END OF BORING @ 15.5'

*
ELEVATION ESTIMATED
FROM CONTOUR PLAN

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

Boring Log GOLF COURSE
PROJECT SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii

Tax Map Key: 3-9-11

HAMMER:

Weight 140 #

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 30 Sheet No. _____ of _____

Driller WALTER LUMA440C Date JUNE 9, 1970

Field Party LUNING, MAESHIRO

Type of Boring AUGER (ACKER ACE) Diam. 4"

Elev. 72' ± * Datum

Drill Bit T.C. DRAG

Water Level	NOT NOTICED				
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Time	_____	_____	_____	_____	_____
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Date	6-2-70				
------	--------	--	--	--	--

Unified Soil Classification	DESCRIPTION	Depth (ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	PENETRATION DATA				
										Standard Penetration Test				
										N (Blows per foot)				
										0	10	20	30	40
(GM)	MEDIUM DENSITY TO DENSE, TAN, SILTY SAND & MUDROCK	0		30-A	-	25	-	-	-					
		5		30-B	-	21	-	-	-					
(ML)	STIFF, TAN, SANDY SILT w/ DECOMPOSED MUDROCK	10		30-C	-	25	-	-	-					
		15		30-D	-	31	-	-	-					
	END OF BORING @ 16.5'													
* ELEVATION ESTIMATED FROM CONTOUR PLAN														

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

Tax Map Key: 3-9-11

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 31 Sheet No. _____ of _____

Driller _____ Date JUNE 4, 1978

Field Party ASATO, MAESHIRO

Type of Boring AUGER (ACKER) Diam. 4"

Elev. 65' ± * Datum

Drill Bit T. C. DRAG

Water Level NOT NOTICED			
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Time	—			
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Date	6-4-70				
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Unified Soil Classification	DESCRIPTION	Depth (ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	PENETRATION DATA				
										Standard Penetration Test				
										N (Blows per foot)				
										0	10	20	30	40
(ML)	MEDIUM, TAN, SANDY SILT w/ MUDROCK	0		31-A	-	27	-	-	-					
		5		31-B	-	29	-	-	-					
(MH)	STIFF, LIGHT BROWN, CLAYEY SILT w/ SAND & DECOMPOSED MUDROCK	10		31-C	-	37	-	-	-					
		15		31-D	-	46	-	-	-					
	END OF BORING @ 16.5'													
* ELEVATION ESTIMATED FROM CONTOUR PLAN														

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

PROJECT _____ SUBDIVISION UNIT NO. 2

LOCATION MAUNALUA, OAHU, HAWAII

TAX MAP KEY: 3-9-11

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 32 Sheet No. _____ of _____

Driller WALTER LUMA440C. Date JUNE 8, 1970

Field Party LUNING, MAESHIRO

Type of Boring AUGER (ACKER) Diam. 4"

Elev. 80' ± * Datum

Drill Bit T. C. DRAG

Water Level	NOT NOTICED			
-------------	----------------	--	--	--

Time	_____	_____	_____	_____
------	-------	-------	-------	-------

Date <u>6-8-70</u>			
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[illegible]

WALTER LUM ASSOCIATES

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION Maunaloa, Oahu, Hawaii

Tax Map Key: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER:

2" STANDARD SPLIT SPOON

BORING NO. 33 Sheet No. of

Driller WALTER LUM ASSOC. Date JUNE 8, 1970

Field Party LUNING, MAESHIRO

Type of Boring AUGER (ACKER) Diam. 4"

Elev. 20' ± * Datum

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time

Date 6-8-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)
0 10 20 30 40

Unified Soil Classification

DESCRIPTION

ELEV. = 20' ± *

Depth (Ft.)

Sampler

Sample No.

Wet Dens. P.C.F.

Water Cont. %

Dry Dens. P.C.F.

Unconf. Comp. P.S.F.

Vane Shear P.S.F.

(ML)

STIFF, TAN.
SANDY SILT
W/ MUDROCK

END OF BORING @ 16'

*
ELEVATION ESTIMATED
FROM CONTOUR PLAN

33-A

29

33-B

29

33-C

30

33-D

31

WALTER LUM ASSOCIATES

3030 WAIALAE AVENUE • HONOLULU, HAWAII 96816 • PHONE 737-7931

Boring Log

PROJECT GOLF COURSE
SUBDIVISION UNIT NO. 2

LOCATION MAUNALUA, OAHU, HAWAII
TAX MAP KEY: 3-9-11

HAMMER:

Weight 140#

Drop 30"

SAMPLER: 2" STANDARD SPLIT SPOON

BORING NO. 34 Sheet No. of

Driller WALTER LUM ASSOC. Date JUNE 8, 1970

Field Party ASATO, MAKAULA

Type of Boring AUGER (CONCORE AB-JR) Diam. 4"

Elev. 100' ± * Datum

Drill Bit T.C. DRAG

Water Level NOT NOTICED

Time

Date 6-8-70

PENETRATION DATA

Standard Penetration Test

N (Blows per foot)
0 10 20 30 40

Unified Soil Classification

DESCRIPTION

ELEV. = 100' ± *

Depth (Ft.)

Sampler

Sample No.

Wet Dens. P.C.F.

Water Cont. %

Dry Dens. P.C.F.

Unconf. Comp. P.S.F.

Vane Shear P.S.F.

(ML-GM)

STIFF, TAN.
SANDY SILT
& MUDROCK

PUKA PUKA ROCK

END OF BORING @ 15.7'

* ELEVATION ESTIMATED
FROM CONTOUR PLAN

Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	Standard Penetration Test N (Blows per foot)
34-A	-	20	-	-	-	
34-B	-	21	-	-	-	23/1.5'
34-C	-	25	-	-	-	
34-D	-	23	-	-	-	35/1.2'

Boring Log

PROJECT GOLF COURSE
 SUBDIVISION UNIT NO. 2
 LOCATION MAUNALUA, OAHU, HAWAII
 TAX MAP KEY: 3-9-11

BORING NO. 35 Sheet No. _____ of _____Driller _____ Date JUNE 4, 1970Field Party ASATO, MAESHIROType of Boring AUGER (ACKER ACE) Diam. 4"Elev. 93' ± * Datum _____Drill Bit T.C. DRAGWater Level NOT NOTICED

Time _____

Date 6-4-70

HAMMER:

Weight 140#Drop 30"SAMPLER: 2" STANDARD SPLIT SPOON

PENETRATION DATA

Unified Soil Classification	DESCRIPTION	Depth (Ft.)	Sampler	Sample No.	Wet Dens. P.C.F.	Water Cont. %	Dry Dens. P.C.F.	Unconf. Comp. P.S.F.	Vane Shear P.S.F.	Standard Penetration Test				
										N (Blows per foot)				
										0	10	20	30	40
(ML)	MEDIUM, TAN, CLAYEY SILT W/ MUDROCK	0		35-A	-	25	-	-	-					
		5		35-B	-	20	-	-	-					
		10		35-C	-	28	-	-	-					
		15		35-D	-	28	-	-	-					
MH	MEDIUM TO STIFF, BROWN, CLAYEY SILT W/SAND													
	END OF BORING @ 16.0'													

*
 ELEVATION ESTIMATED
 FROM CONTOUR PLAN

GOLF COURSE SUBDIVISION - UNIT 2

TABLE I A - SUMMARY OF LABORATORY TEST RESULTS

BORING NO.	22	27	29	35
SAMPLE NO.				C
DEPTH BELOW SURFACE	SURFACE	SURFACE	SURFACE	10'-11.5'
DESCRIPTION	BROWN SILTY SAND W/GRAVEL	TAN BROWN SANDY SILT	BROWN CLAYEY SILT W/SAND	BROWN CLAYEY SILT W/SAND
GRAIN-SIZE ANALYSIS (% Passing)				
Sieve				
1"	100		100	
1/2"	91.0		100	
#4	81.4		96.0	
#10	73.4		93.5	
#20	64.5		90.8	
#40	53.9		87.6	
#100	40.2		81.3	
#200	34.0		74.7	
ATTERBERG LIMITS				
Air Dried or Natural	NATURAL	NATURAL	NATURAL	NATURAL
Liquid Limit	62	49	55	56
Plastic Limit	46	36	37	33
Plasticity Index	16	13	18	23
Dilatancy	QUICK	QUICK	SLOW	SLOW
Toughness	SLIGHT	SLIGHT	MEDIUM	SLIGHT-MED
Dry Strength	SLIGHT	SLIGHT	MEDIUM	MEDIUM
UNIFIED SOIL CLASSIFICATION	SM	ML	MH	MH
APPARENT SPECIFIC GRAVITY	2.72	2.87	2.89	
EXPANSION AND CBR TESTS (Surcharge-51 P.S.F.)				
Molding Moisture, %	40.8	22.9	26.6	
Molding Dry Density, P.C.F.	74.0	94.6	84.7	
Swell upon saturation, %	1.7	1.5	1.2	
CBR at 0.1" Penetration	3.1	19.0	23.0	
MOISTURE-DENSITY RELATIONS OF SOILS (AASHTO T-180-57 Method <u> </u>)				
Dry to Wet or Wet to Dry	A	A	A	
Max. Dry Density (P.C.F.)	DRY TO WET 76.0	DRY TO WET 93.8	DRY TO WET 88.6	
Optimum Moisture (%)	37.4	26.0	29.1	

REMARKS:

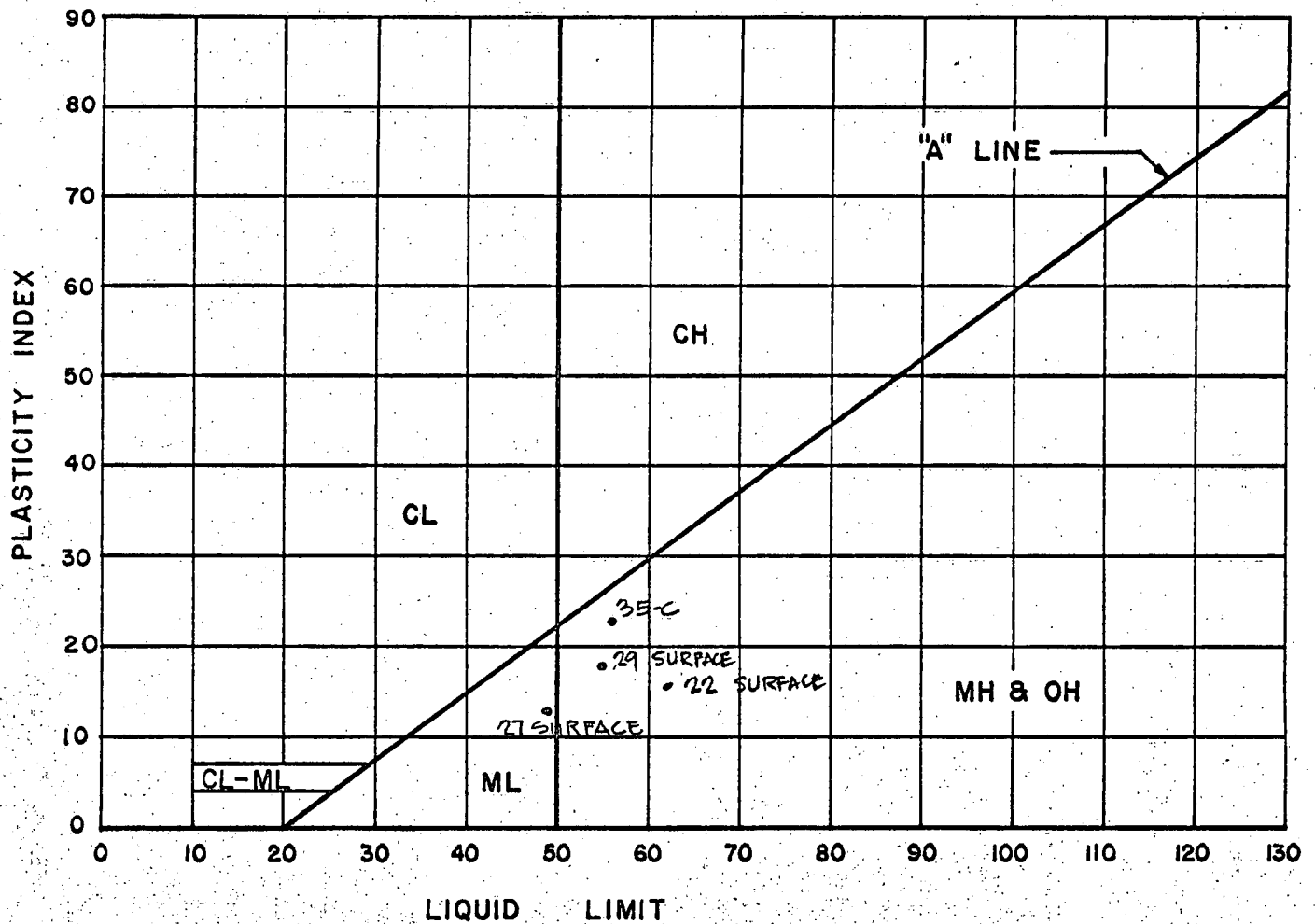
WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

Date _____ By _____

PLASTICITY CHART

PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII



WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

DATE 8-24-70 BY C.M.

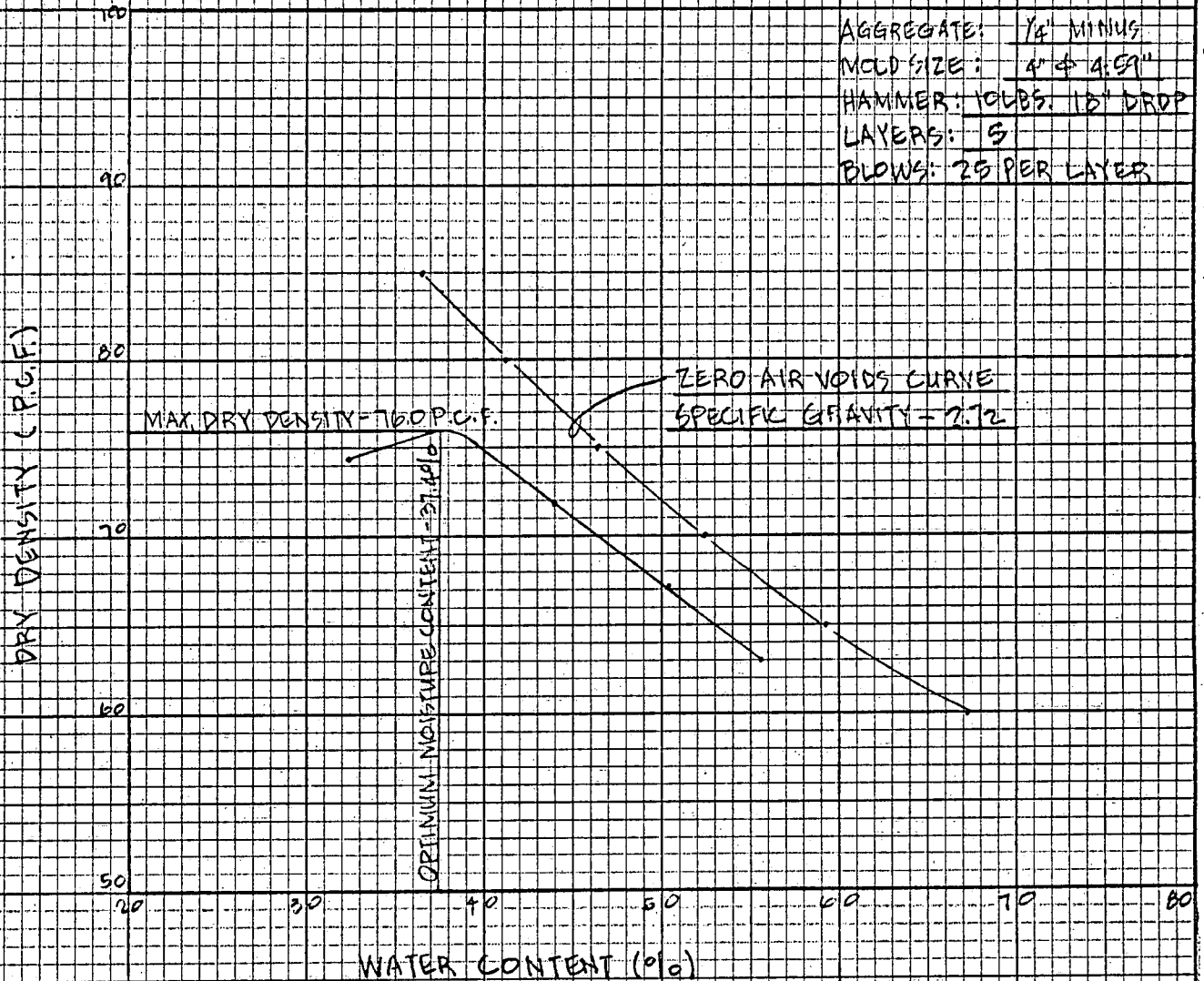
MOISTURE - DENSITY CURVE (AASHTO T-180-57 METHOD A)

PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA OAHU, HAWAII

SAMPLE NO: 22 - SURFACE

SAMPLE DESCRIPTION: BROWN SILTY SAND W/ GRAVEL



DATE: 7-25-10 BY: S.T.

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGRS.

MOISTURE-DENSITY CURVE (AASHTO T-180-57, METHOD A)

PROJECT: GOLF COURSE SUBDIVISION-UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII

SAMPLE NO.: 27 SURFACE

SAMPLE DESCRIPTION: TANNISH BROWN SANDY SILT

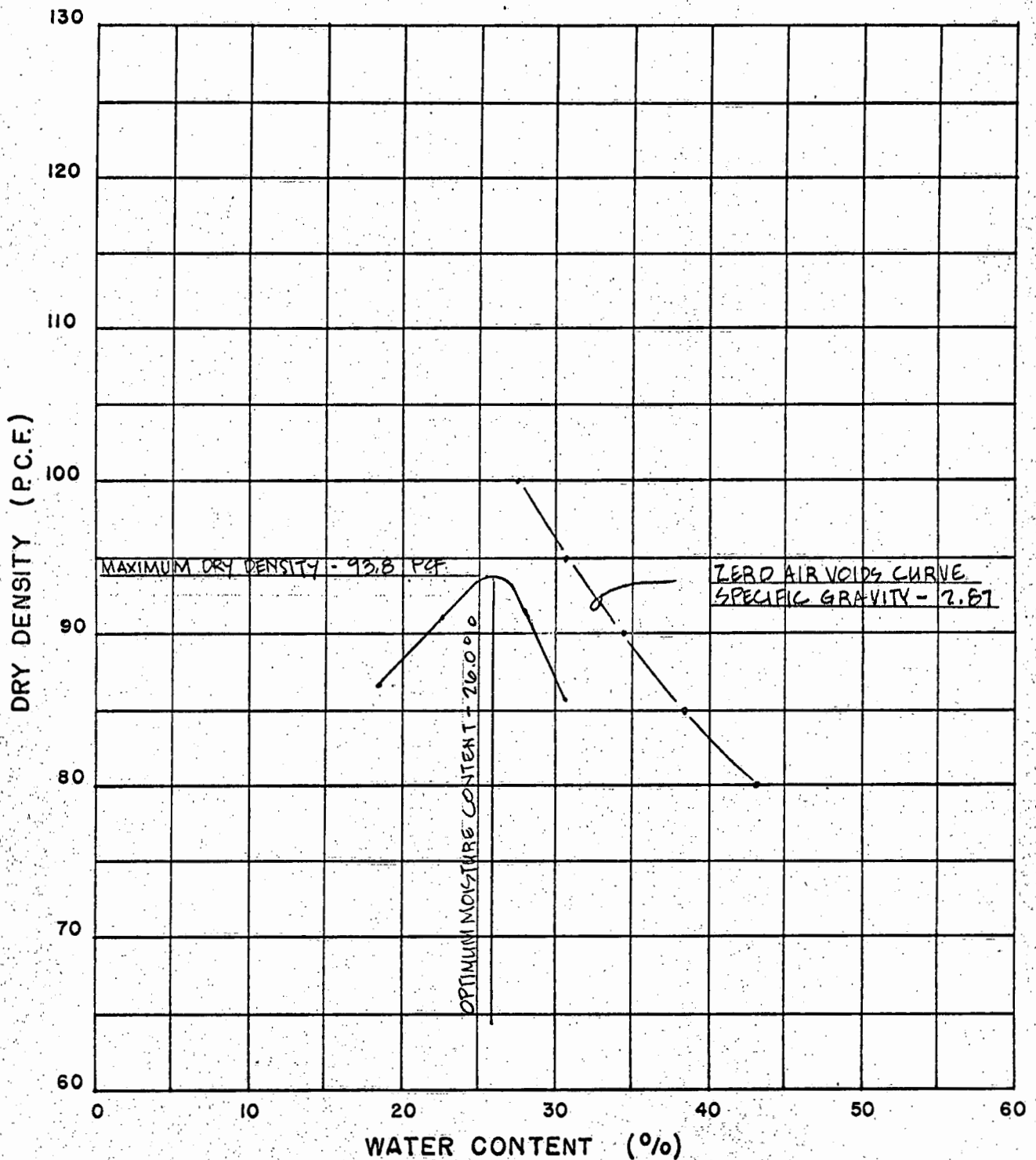
AGGREGATE: 1/4" MINUS

MOLD SIZE: 4" x 4.59"

HAMMER: 10 LBS 18" DROP

LAYERS: 5

BLOWS: 25 PER LAYER



WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

DATE 9-1-70 BY B.T.

MOISTURE-DENSITY CURVE (AASHTO T-180-57, METHOD A)

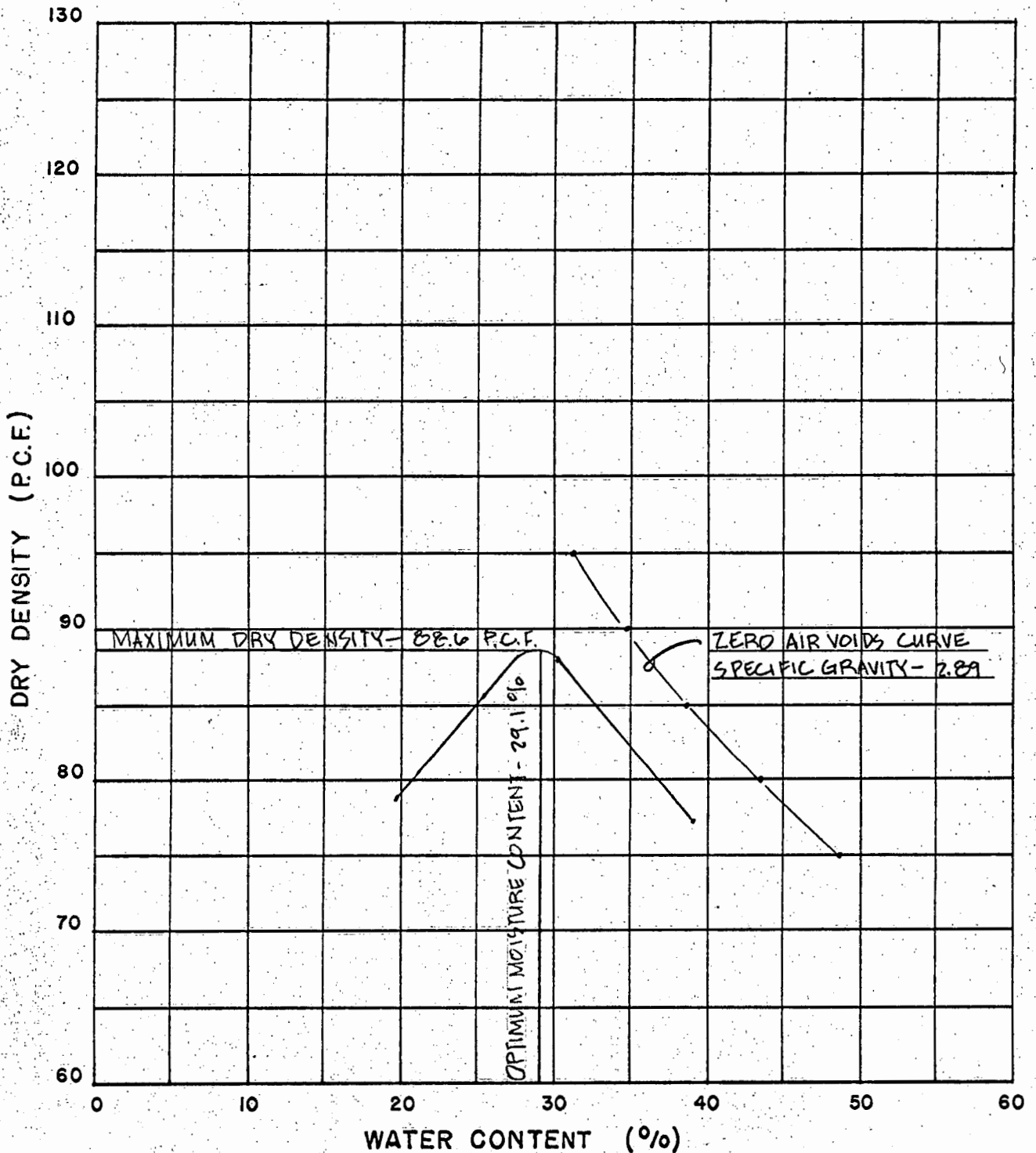
PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII

SAMPLE NO.: 29 SURFACE

SAMPLE DESCRIPTION: BROWN CLAYEY SILT WITH SAND

AGGREGATE: 1/4" MINUS
 MOLD SIZE: 4" ϕ 4.75"
 HAMMER: 10 LBS. 18" DROP
 LAYERS: 5
 BLOWS: 25 PER LAYER



WALTER LUM ASSOCIATES, INC.
 CIVIL, STRUCTURAL, SOILS ENGINEERS

DATE 8-8-70 BY S.T.

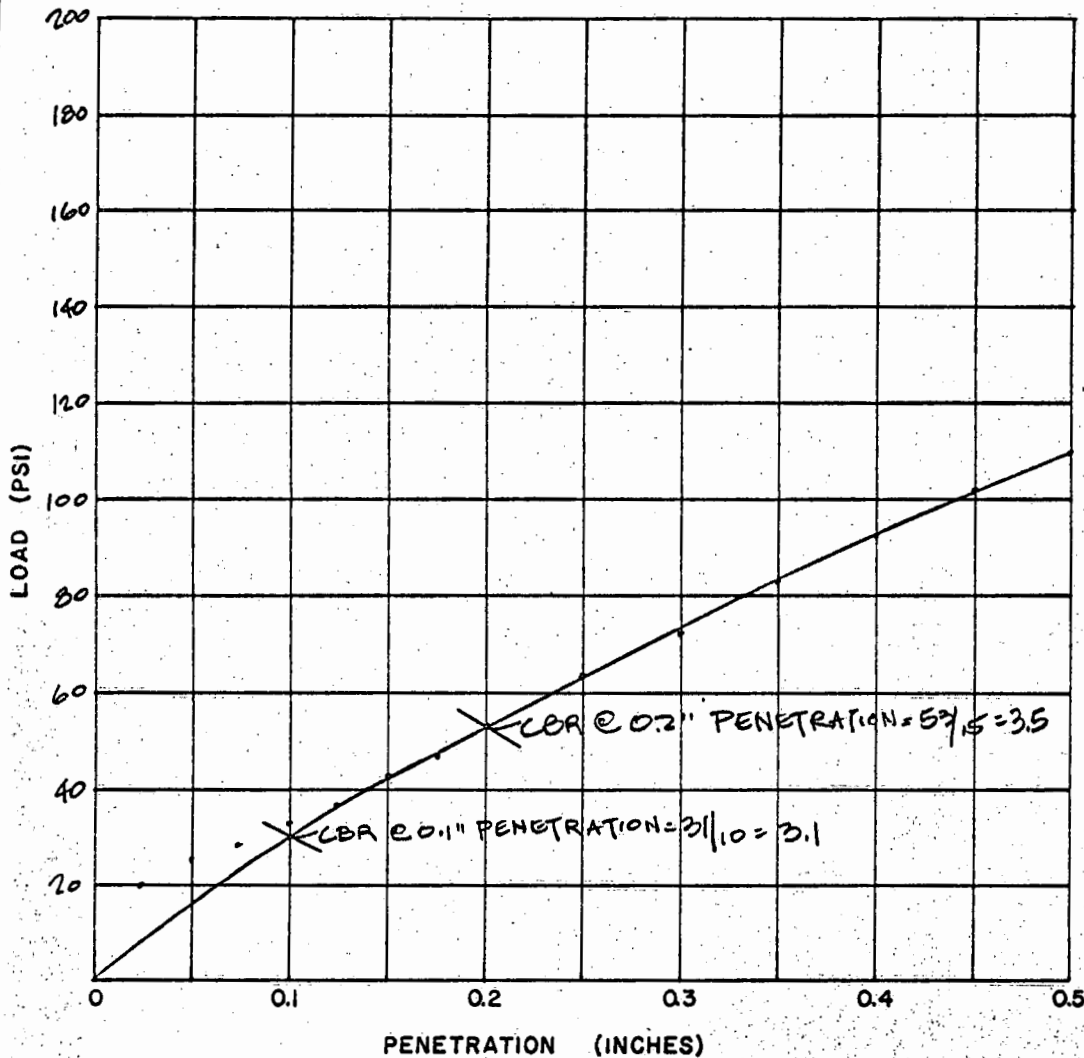
CBR TEST

PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII

SAMPLE NO: 22 - SURFACE

SAMPLE DESCRIPTION: BROWN SILTY SAND W/ GRAVEL



CBR PENETRATION DATA

PENETRATION (INCHES)	LOAD (LBS)	LOAD (PSI)
0.025	60	20
0.050	75	25
0.075	85	28
0.100	100	33
0.125	110	37
0.150	130	43
0.175	140	47
0.200	160	53
0.250	190	63
0.300	220	73
0.350	250	83
0.400	280	93
0.450	305	102
0.500	330	110

AGGREGATE 1/4" MINUS
HAMMER WEIGHT 10 LBS
HAMMER DROP 18"
No. OF BLOWS 56
No. OF LAYERS 5

TEST RESULTS:

MOLDING MOISTURE, %. 40.2

MOLDING DRY DENSITY, P.C.F. 14.0

CBR @ 0.1" PENETRATION 3.1

DATE 7-28-70 BY C.M.

DATE 8-3-70 BY S.T.

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

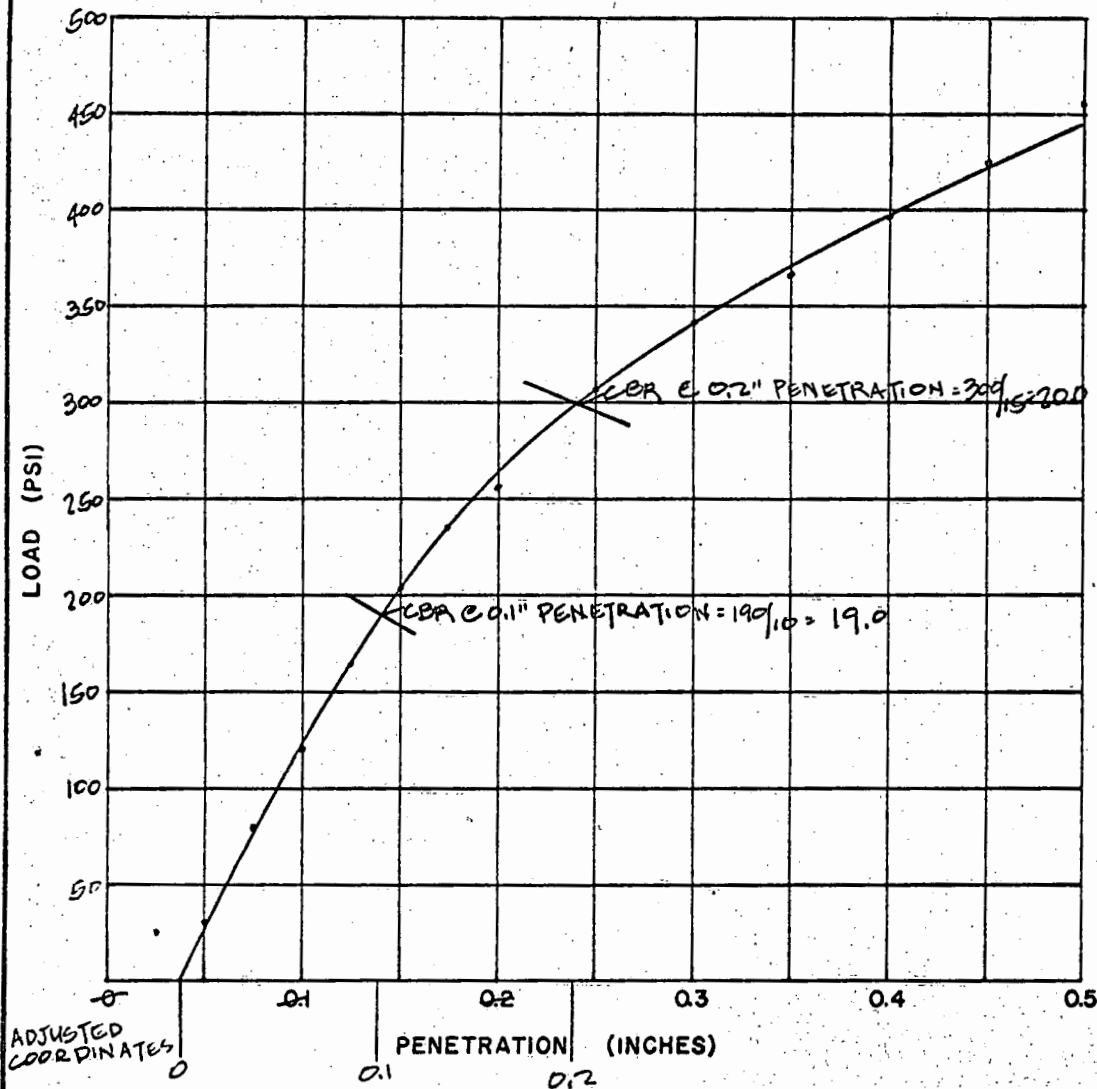
CBR TEST

PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII

SAMPLE NO: 27 SURFACE

SAMPLE DESCRIPTION: TAN-BROWN SILTY SAND



CBR PENETRATION DATA

PENETRATION (INCHES)	LOAD (LBS)	LOAD (PSI)
0.025	75	75
0.050	90	30
0.075	240	80
0.100	360	120
0.125	495	165
0.150	610	203
0.175	705	235
0.200	775	258
0.250	920	307
0.300	1025	342
0.350	1105	368
0.400	1190	397
0.450	1275	425
0.500	1365	455

AGGREGATE 1/4" MINUS
HAMMER WEIGHT 10 LBS
HAMMER DROP 18"
No. OF BLOWS 56
No. OF LAYERS 5

TEST RESULTS:

MOLDING MOISTURE, % 22.9
MOLDING DRY DENSITY, P.C.F. 94.6
CBR @ 0.1" PENETRATION 19.0

DATE 8-4-70 BY R.M.

DATE 8-10-70 BY S.T.

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

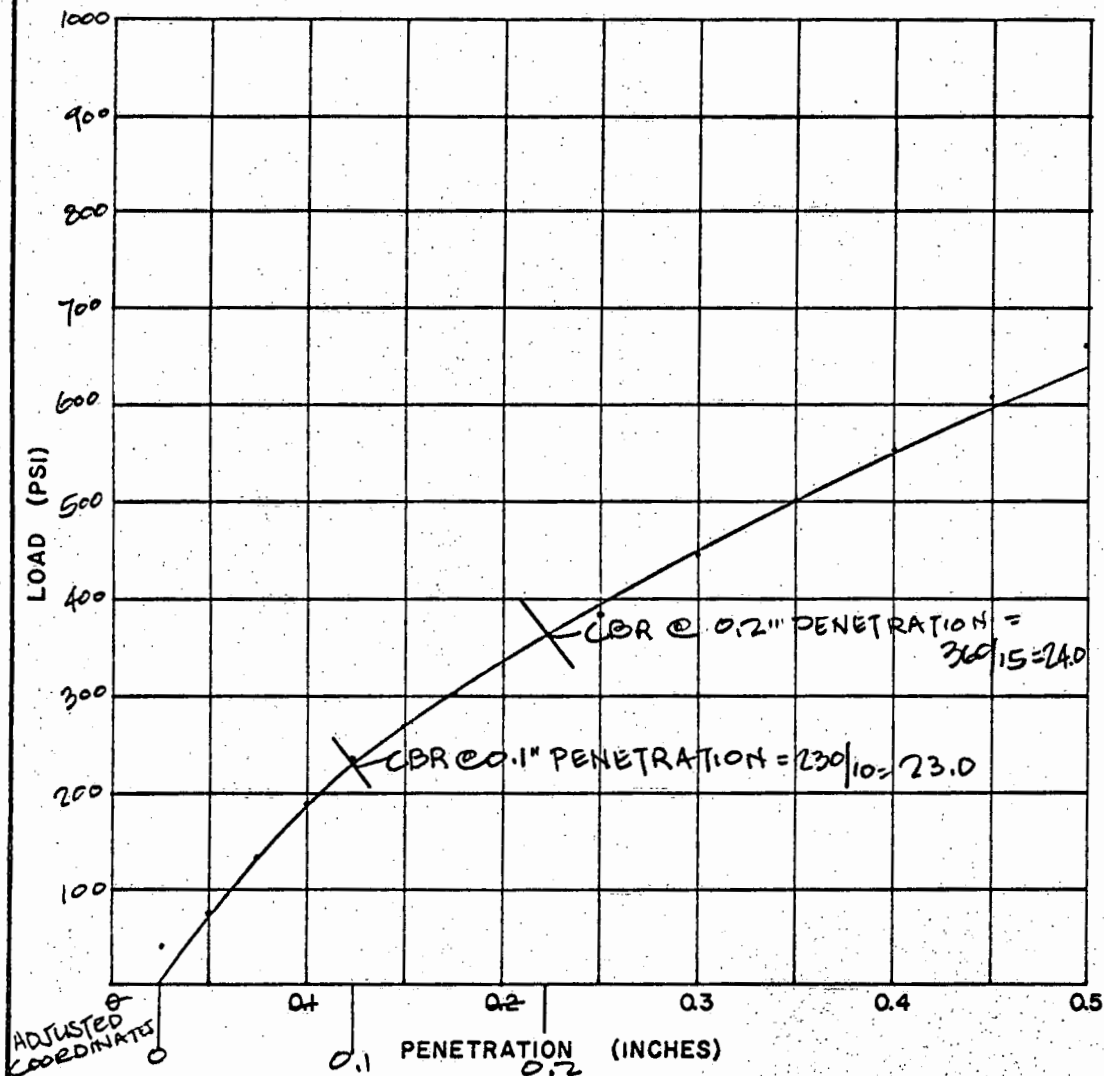
CBR TEST

PROJECT: GOLF COURSE SUBDIVISION - UNIT 2

LOCATION: MAUNALUA, OAHU, HAWAII

SAMPLE NO: 29 SURFACE

SAMPLE DESCRIPTION: BROWN CLAYEY SILT W/SAND



CBR PENETRATION DATA

PENETRATION (INCHES)	LOAD (LBS)	LOAD (PSI)
0.025	110	37
0.050	220	73
0.075	390	130
0.100	570	190
0.125	710	237
0.150	810	270
0.175	910	303
0.200	1000	333
0.250	1160	387
0.300	1340	447
0.350	1505	502
0.400	1670	557
0.450	1830	610
0.500	1990	663

AGGREGATE 1/4" MINUS
HAMMER WEIGHT 10 LBS
HAMMER DROP 18"
No. OF BLOWS 56
No. OF LAYERS 5

TEST RESULTS:

MOLDING MOISTURE, % 26.6
MOLDING DRY DENSITY, P.C.F. 84.1
CBR @ 0.1" PENETRATION 23.0

DATE 7-28-70 BY C.M.
DATE 8-3-70 BY S.T.

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS

GENERAL TESTING METHODS

EXPLORATORY BORINGS AND SAMPLING

Method for soil investigation and sampling
by auger borings (Tentative)

ASTM Designation: D 1452-63T

Method for thin wall tube sampling of
soils (Tentative)

ASTM Designation: D 1587-63T

Method for penetration test and split
barrel sampling of soils (Tentative)

ASTM Designation: D 1586-64T

LABORATORY TESTING

Grading Analysis

Sieve analysis of fine and coarse
aggregates

AASHTO Designation: T 27-60

Amount of material finer than
No. 200 sieve in aggregate

AASHTO Designation: T 11-60

Atterberg Limits

Determining the liquid limit of soils
Modified as follows: Substitute
Casagrande grooving tool. Tests
conducted from natural moisture
content unless noted otherwise.

AASHTO Designation: T 89-60

Determining the plastic limit of soils

AASHTO Designation: T 90-56

Calculating the plasticity index of
soils

AASHTO Designation: T 91-54

Specific Gravity

Specific gravity of soils
Modified as follows: 500 ML Pycnometer

AASHTO Designation: T 100-60

Expansion and CBR Tests

Expansion test and California Bearing
Ratio (CBR)

Section VIII - TM 5-530
"Materials Testing" by Headquarters,
Dept. of the Army

Compaction Test

Moisture-Density relations of soils
using a 10# rammer and an 18" drop

AASHTO Designation: T 180-57

Unified Soil Classification

Designation E-3 from "Earth
Manual" by the United States
Department of the Interior
Bureau of Reclamation

GENERAL TESTING METHODS

Consolidation Test

Laboratory Shear Test

Laboratory shear test using
the Torvane

Chapter IX
"Soil Testing for Engineers"
by T. William Lambe
The Massachusetts Institute
of Technology

Brochure by Soiltest, Inc.

LIMITATIONS

In general, soil formations are commonly erratic and rarely uniform or regular. The boring logs indicate the approximate subsurface soil conditions encountered only at the drill holes where the borings were made at the times designated on the logs and may not represent conditions at other locations or at other dates. Soil conditions and water levels may change with the passage of time and construction methods or improvements at the site.

During construction, should subsurface conditions much different from those in the borings be observed, encountered, or otherwise indicated, we should be advised immediately to review or reconsider our recommendations in light of the new developments.

Our professional services were performed, findings obtained and recommendations prepared in accordance with generally accepted engineering practices. This warranty is in lieu of all other warranties expressed or implied.

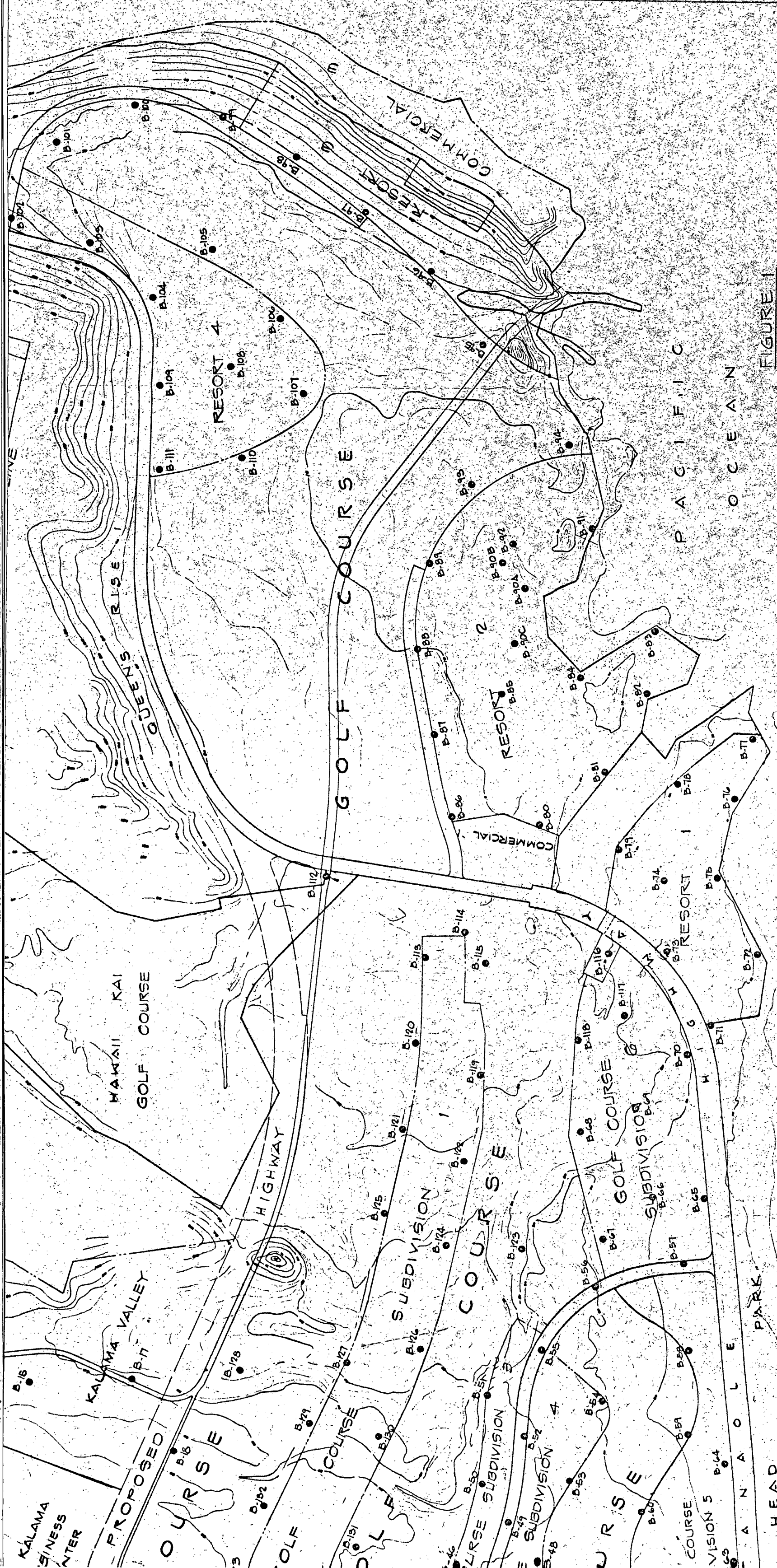
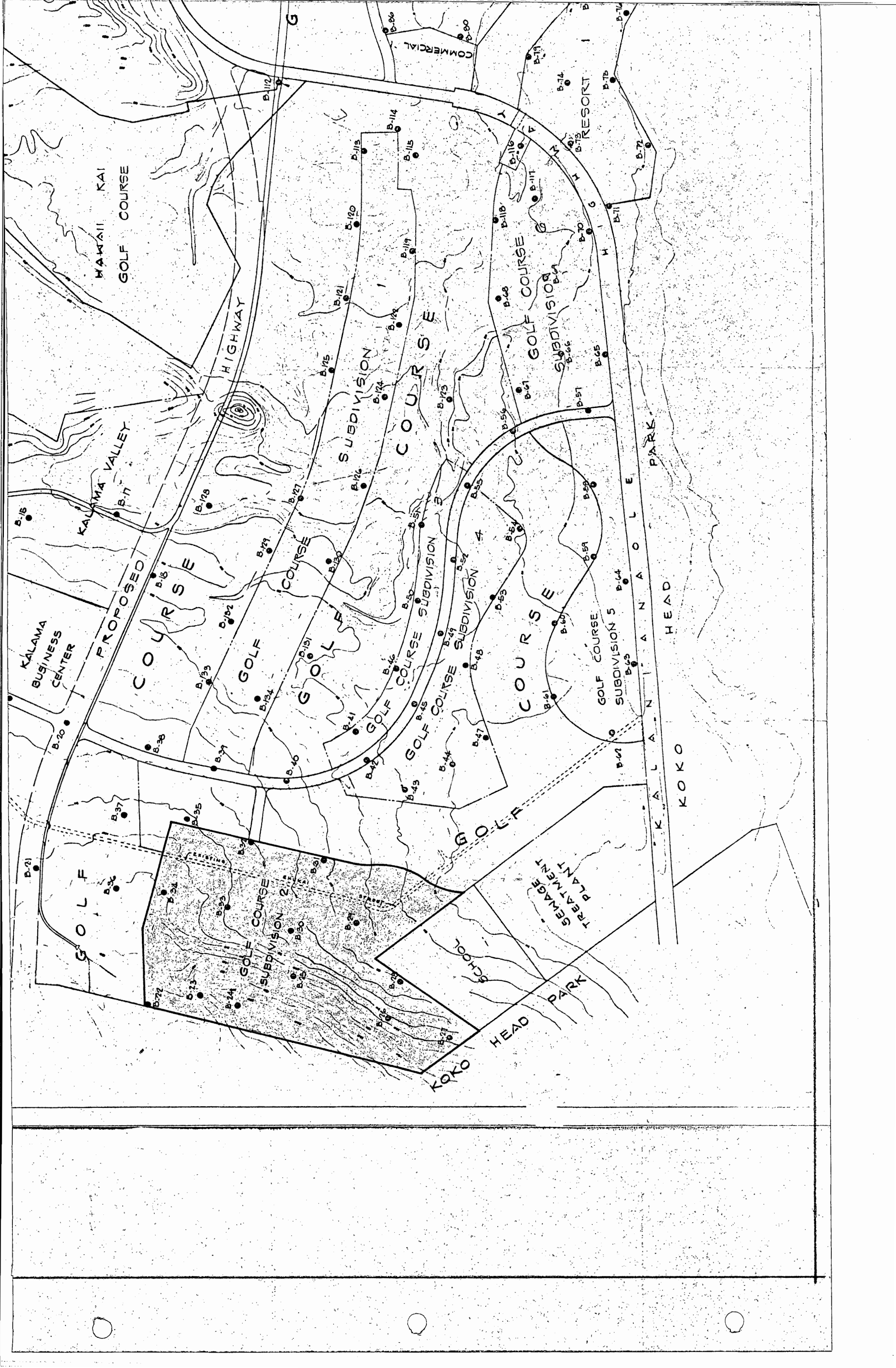
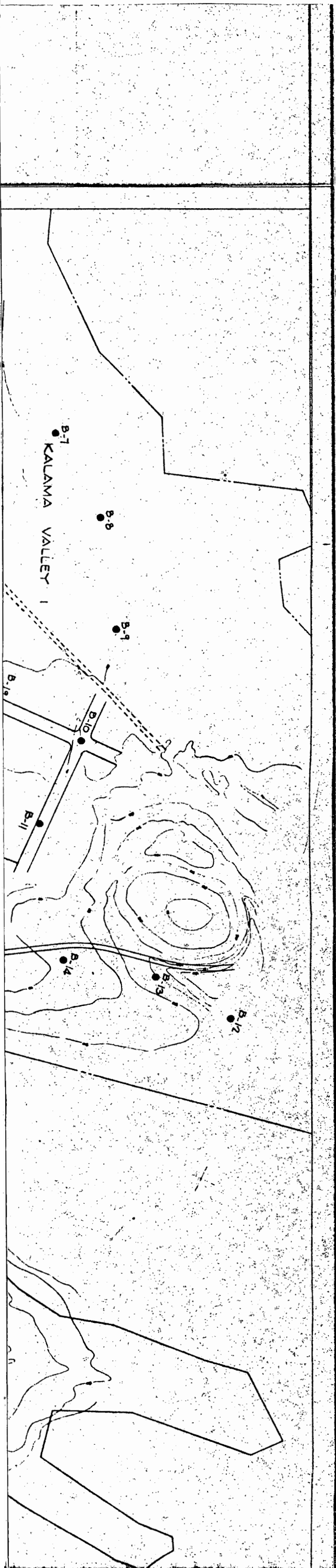
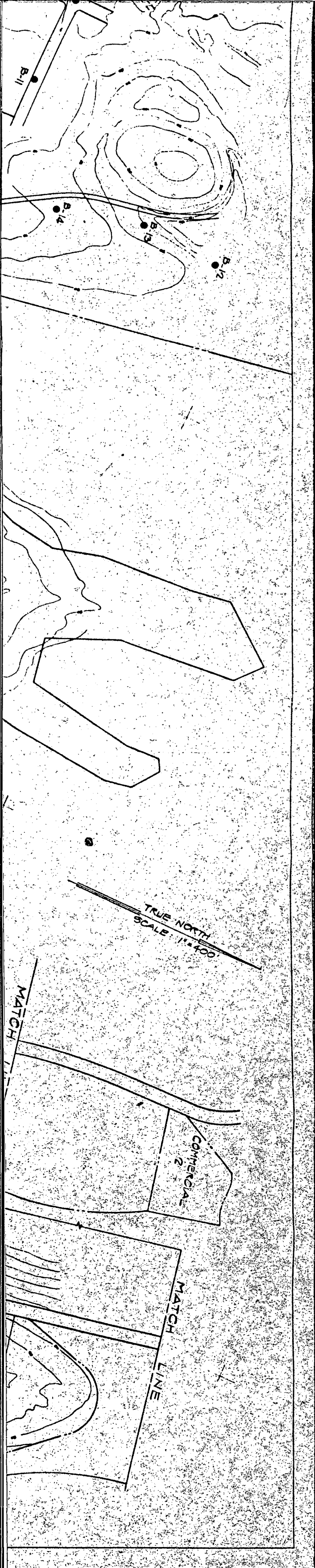


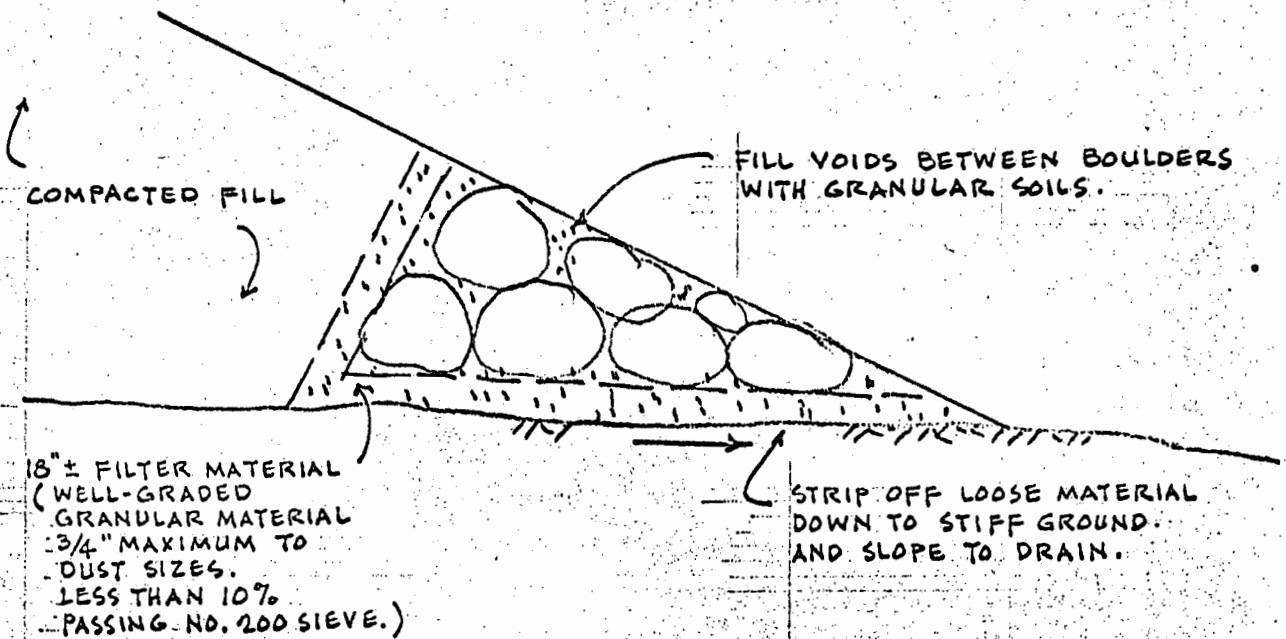
FIGURE 1

BORING LOCATION PLAN	
GOLF COURSE SUBDIVISION UNIT No. 2	
MAUNALUA, OAHU, HAWAII	
TAX MAP KEY: 9-9-11	
Dr. _____	WALTER IUM ASSOCIATES, INC.
Date: 9/10	303 KALANANUOUE
Rev. _____	CIVIL ENGINEERS
	PHONE 727-7831
	Sheet _____ of _____









SECTION

NOT TO SCALE

FIGURE 2

PROPOSED BOULDER FILL

GOLF COURSE SUBDIVISION UNIT No. 2

MAUNALUA, OAHU, HAWAII

TAX MAP KEY: 3-9-11

WALTER LUM ASSOCIATES, INC.
CIVIL, STRUCTURAL, SOILS ENGINEERS